

## Time-Dependent Corrosion Wastage Model for Wrought Iron Structures

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### ABSTRACT

Damage due to atmospheric corrosion on metal structures is a significant aspect for both the design of new constructions and the maintenance of existing buildings. This problem is particularly felt for nineteenth-century "wrought iron" constructions, because of both lack of proper maintenance and architectonic value. The main objective of the current paper, framed within a more comprehensive research project, is to provide a time-dependent model able to predict the corrosion wastage thickness on historical metal structures as a function of the protection coating life variability and its renovation cycles. Average damage curves, calculated on a sample of 20 buildings experimentally monitored for 20 years, were taken as literature references to calibrate the model, based on the hypothesis that the durability, due to phosphorus content, of historical wrought irons are between that of recent mild carbon steels and that of weathering steels. The reference damage curves for two environmental conditions (marine and urban-industrial) and two different materials (mild carbon steel and weathering steel) were interpolated, fitted and extended to 125 years. A comparison was made between experimental damage curves and some significant models from literature and the percentage error with respect to the tolerance and confidence intervals of the reference damage curves was discussed. Results definitely confirm a substantial difference between experimental values and those

predicted by literature models. As an application example, the model was applied to estimate the remaining life of the metal structural elements of the Umberto I Gallery in Naples, one of the most significant monuments of the largest city in the Southern Italy.

## **KEYWORDS**

Wrought iron; atmospheric corrosion; corrosion rate; metallic materials; damage modelling; durability.

## **Introduction**

It is well known that the durability of metal structures is strongly influenced by damage due to corrosion (Rozenfeld, 1972; Barton, 1976; Kucera et al., 1987; Costa et al., 1989; Morcillo et al., 1998; Tidblad et al., 1998 and 2018; Leygraf and Graedel, 2000; Dean et al., 2010; Landolfo et al., 2010). Even nowadays, corrosion of metals has a non-negligible economic impact, that is evident if one looks at the costs sustained in this field by the most advanced and developed countries. For the sake of example, the U.S. economy estimated a corrosion protection cost of almost \$300 billion per year at current prices (ASM, 2000). The inappropriate and non-continuous maintenance of metal structures has a substantial effect on the economy, because reconstruction costs are generally greater than periodic costs for maintenance (Davis, 2000; Teylor, 2013; Drisko and Jenkins, 1998; Dolgikh et al., 2014). For this reason, control and monitoring are two key aspects for the design of new constructions and, above all, for the safeguard of historical buildings.

The research described in the present paper investigates prediction models regarding corrosion propagation due to the lack of adequate information regarding this issue provided by current codes. In fact, in Section titled Codes overview a short synthesis of the main codes in the field

of metal corrosion is given with the aim to discuss the state-of-the art and its weaknesses. Moreover, in the same section it is shown that codes do not specifically give instructions to predict the corrosion depth for both newly and already built structures.

The research goal discussed in this paper is to optimize a time-dependent model to predict corrosion propagation in wrought iron members (Guerrieri et al., 2005; Landolfo et al. 2007; Di Lorenzo et al 2016, 2017a and 2017b), so as to use corrosion damage as a yard stick to determine the material remaining mechanical strength. The focus on wrought iron is due to many reasons. Firstly, in literature there is a lack of corrosion prediction models for this material, which is also neglected by current codes. The Section titled Literature overview deals with an overview of these prediction models in order to compare them with experimental data on corrosion of wrought iron structures. The general feature of these models is a crucial issue for the corrosion model application. In addition, a great deal of information is available regarding short and mid-term periods. Therefore, information on long-term exposure (10 - 20 years) is much less abundant and no consistent data are available for exposure times over a period of 50 years (Morcillo et al., 2011).

Secondly, corrosion propagation affects ferrous alloys differently from one to another even under the same boundary conditions, i.e. air quality and hygroscopic parameters, environmental and meteorological circumstances, etc. For example, the primary critical relative humidity for metal surfaces without corrosion seems to be virtually the same (around 60%) for all metals (Leoni, 1984), but the secondary humidity values (i.e. between 70% and 80%) (Roberge et al., 2002) vary quite widely (Syed, 2006). For this reason, the maintenance process should be done on the basis of the chemical composition of structural materials.

Thirdly, this category of ferrous alloys was widely used in the past as basic materials for metal structures. In fact, many historical buildings and bridges, also with very important architectural

and structural value (i.e. galleries, roofs and memorials), were made of wrought iron alloys. This use reached its peak in 1860s. Very well-known examples of wrought iron alloys structures are the Britannia Bridge in North Wales (1850), the Paddington Station in London (1854), the St. Pancras Station in London (1868) and the Eiffel Tower in Paris (1889). Wrought iron, with its high tensile strength, was widely used during the Railway Age and also when the shipbuilding practice achieved fabricated structures by riveting rolled wrought iron sections each to other. It was also commonly used in civil constructions and, in particular, in building bridges for railways (Lee, 2008). In the framework of the latter issue, due to both significant architectonic - cultural value and age of historical buildings, monitoring the decay of metal elements due to the atmospheric corrosion is a preliminary and essential phase for designing proper restoration interventions. The reduction of metal thickness due to corrosion increases the structure vulnerability level towards seismic or wind actions foreseen by the current standards. This is even worse for structures subjected to cyclic loads, where corrosion phenomenon may produce a significant reduction in fatigue strength, mainly in zones with high stress concentrations, such as holes, notches and connections (Gelfi and Solazzi, 2005; AA.VV., 2009).

The Section titled The baseline experimental database describes experimental results estimated on wrought iron structures and used to calibrate the new model proposed in the current research work (Guerrieri, 2005; Landolfo et al., 2007). The experimental dataset was compared to some significant literature models (Landolfo at al., 2005), specifically summarized in the final Appendix. In conclusion, the Section titled A time-dependent damage model for predicting corrosion thickness wastage in wrought iron elements comments the peculiarity of the corrosion propagation model proposed, while the Section titled Application of the proposed corrosion wastage model to a case study describes an application of the proposed model.

## **Codes overview**

Regarding codes, even though corrosion is often a cause of structural failure (NACE, 1991; Elliott, 2003), and structures made of wrought iron are generally very sensitive to corrosion phenomena, there were no significant provisions concerning a corrosion problem forecast, particularly for existing buildings. In the Italian structural code D.M. 2005 (2005), corrosion is expressly included as one of the loads acting on constructions. Corrosion is classified as a type of entropic load, which comprises deteriorating actions, caused by natural degradation mechanisms in materials and environmental loads and, thus, affecting structural integrity. However, few and insufficient indications are given to predict the corrosion phenomena. Current codes, such as EN 1993-1-1 and EN 1993-1-4 (2005), which directly refers to EN 1990:2002/A1 (2005), give only general principles regarding the protection of steel buildings from possible corrosion damage causes. They provide only general recommendations and basic principles that mainly concern the use of protective coating systems, the choice of corrosion resistant materials and structural redundancy strategies (Landolfo et al., 2010). A quantitative measurement of environmental corrosivity and its classification is provided by many different provisions, such as EN ISO 9223, 9224, 9225 and 9226 (1992), EN 12500 (1998 and 2000) and EN ISO 12944-2 (2001). In particular, depending on the degree of corrosiveness, EN 12500 (1998) defines different typologies of atmospheres, whereas EN ISO 9223 (1992) provides a corrosivity classification system to assess the influence of various factors on the damage process. This classification considers both the level of corrosive impurities, characterised by the presence of sulphur dioxide and chloride particles, and the “Time of Wetness” (TOW), estimated as the number of hours when relative humidity and temperatures exceed 80% and 0°C, respectively. This methodology is quite limited in terms of accuracy and precision. Indeed,

the atmospheric parameters conditioning the corrosivity classification do not include the effects of potentially important corrosive pollutants and impurities. Moreover, the effects of wind speed, exposure angle and sheltering (natural and artificial obstacles) are not properly accounted for. In EN ISO 14713 (1999), specific recommendations are provided for each corrosivity class with respect to different coating typologies.

As an alternative to the methodology proposed by the ISO standards (i.e. EN ISO 9223, 9224, 9225 and 9226, 1992, EN ISO 8044, 1999, EN 12500, 1998 and 2000, EN ISO 12944-2, 2001), it is possible to classify atmosphere corrosivity by using the PACER LIME algorithm (Fink, 1980). This algorithm measures the expected corrosion damage related to various parameters opportunely evaluated. A recent example of an improved standard for corrosion assessment is given by the current Italian Ministerial Decree of Public Works (NTC, 2008), which considers atmospheric corrosion as an entropic nature action.

While the codes suggest environmental measurements in order to estimate air, temperature, humidity, wind speed and direction, atmospheric precipitation frequency and exposure to solar and ultraviolet radiation, because of their direct influence on the corrosion phenomenon, they do not give references to any models able to estimate the corrosion depth. Contrary, corrosion predictive models are able to accomplish this goal. The need to have these models was confirmed by research works on wrought iron structures developed in the more comprehensive European research projects devoted to both investigate and protect historical buildings (PROHITECH, 2004, 2017) and to evaluate the durability of constructions (COST C25, 2006).

### **Literature overview**

Literature references in the field of Civil Engineering lack corrosion models appropriately dedicated to historical buildings. Contrary, several models concerning the damage evaluation

produced by the atmospheric corrosion are mainly available in the mechanical and naval fields. They follow different approaches depending on the objectives of the model itself.

Simillion et al. (2014) recognized two approaches, namely heuristic and deterministic, to study the damages provoked by corrosion.

Heuristic models referred to experiments and represent the most intuitive approach to predict corrosion. They are also the most diffused methods followed by a lot of researchers (Kromp, 1987; Svensson and Johansson, 1993; Feliu and Morcillo, 1993; Mendoza and Corvo, 1999 and 2000; Corvo et al., 2008 and 2005; Karaka, 2013; Morcillo et al., 2013 and Fluente et al., 2007 and 2011. The heuristic approach is based on a regression of experimental data. Generally, it is possible to divide the models belonging to this approach into two categories: the 1<sup>st</sup> level models, founded on laws of physics and chemistry, that interpret the phenomenon starting from the parameters on which they are based on; the 2<sup>nd</sup> level models, obtained from observation and interpolation of experimental data, opportunely examined from a statistical point of view (Landolfo et al., 2005; Kucera, 2004). These are based on damage curves that predict the thickness of corrosion wastage over time. The first set of models interprets the corrosion phenomenon based on the causes generating the problem. Such models, deduced theoretically or experimentally, are directly correlated to corrosive atmospheres characterised by the main influence parameters.

The second models, instead, simulate the effects induced from corrosion over time, in terms of either corrosion depth or thickness loss, without referring directly to the corrosive atmosphere and, therefore, to the causes generating the damage. Among the 2<sup>nd</sup> level models, linear, linearized and non-linear models are available. When 2<sup>nd</sup> level models available in literature are examined and compared to each other, it is apparent that some of them model the corrosion protection system (*cps*), in particular predicting its effectiveness loss, where some others do

not consider the protection option. There is a great variety of different corrosion prediction models, so that a literature overview is considered as an important issue to define an analytical procedure used to implement the new model proposed in the Section titled A time-dependent damage model for predicting corrosion thickness wastage in wrought iron elements. In addition, it is essential to have an outline of the key assumptions (specific experimental dataset or explicit kinds of steel or different atmospheres) in literature models (Kusmieriek and Chrzescijanska, 2015) compared with those of the new model herein proposed.

In this section, some of the landmark models are shortly commented with the aim to clarify the mathematical basis of the new proposed model. In Appendix 1, formulations of the models compared in the Section titled The baseline experimental database to the experimental dataset used in this work are reported. Most of these models were developed for mechanical and naval engineering and they were calibrated using experiments based on immersion either in water or in a marine atmosphere. Models calibrated on experimental data of immersed corroded steel are of crucial importance, since they represented pioneering applications in this field. Some of these models were examined by Southwell et al. (1979) and Melchers (1999). Southwell et al. discussed a 1<sup>st</sup> level model of the corrosion loss for immersed mild steel in the tropics. Based on the assumption that long-term corrosion was influenced mainly by anaerobic conditions, this model was represented by a linear expression with a constant at time zero and a Steady State corrosion rate. Melchers (1999), based on the experimental data published in Melchers (1998) and on the theoretical approach exposed in Melchers (1987), applied the Southwell et al.'s linear equation to fit experimental data related to the thickness loss expressed as a function of the time (in years). It also proposed a 2<sup>nd</sup> level model for mild and low alloy steels in immersion conditions with a bi-linear equation directly based on experimental data. The model



was improved by the same author (Melchers, 2003a and 2003b) and it was applied to the marine immersion corrosion of carbon content of low alloy steels.

Three relevant models similar to that of Melchers (1999) were developed by Yamamoto and Ikegami (1996; 1998a, 1998b), who were amongst the first researchers to establish a model that took into account the degradation of painting coatings, as well as the generation and the progress of the pitting points, Guedes-Soares and Garbatov (1999), who tested a ship hull girder subjected to corrosion and Paik et al. (1998, 2004), Paik and Kim (2013) and Mohd and Paik (2013), who worked on seawater ballast tank structures of ships.

The Sarveswaran's model (1<sup>st</sup> level model in 1996) was one of the first methods calibrated on environmental corrosion (i.e. non-immersed elements). The model was founded on the varying thickness loss model using a percentage thickness loss calculation. Since the model is evaluated referring to a I-beam profile, it is linear and does not consider the *cps*. In addition, it was only indirectly a time-dependent model. On the contrary, directly compared with the new model proposed in this paper are three models calibrated in atmosphere by Albrecht and Hall (2003), Klimesmith et al. (2007) and International Cooperative Programme (ICP). Albrecht and Hall gave some adjustments to the EN ISO 9224 (1992) corrosion laws, proposing a new bi-linear model accounting for a modified corrosion rate during the first year of exposure and a steady state during the subsequent years. Klimesmith et al. (2007) developed a model for the atmospheric corrosion of carbon steel, zinc, copper and aluminum, taking into account the effects of four environmental variables, namely TOW, sulfur dioxide, salinity and temperature. Finally, the dose-response model was developed within the International Cooperative Programme (ICP) on "Effects on Materials, including Historic and Cultural Monuments", in the framework of the UN ECE convention on long range trans-boundary air pollution. The last model was formulated on different metal materials and it was based on both long-term

exposures and trend analysis founded on repeated one-year measurements, taking also into account unsheltered or sheltered exposure (Landolfo et al., 2010; Kucera, 2004). The new model proposed in this paper is directly inspired to the 2<sup>nd</sup> level model previously described. For the sake of completeness, it is necessary to speak about deterministic models. They are basically founded on corrosion mechanisms (connected to the chemical and molecular structure of materials) and are multi-scale models generally more complex than others (Simillion et al., 2014). GILDS (Gas - Interface - Liquid - Deposition - Electrode - Solid) are some examples of these multi-scale models. They evaluate the kinetics of the corroding system, including interactions with the gaseous and liquid environments (Venkatraman et al., 2011; Tidblad and Graedel, 1996; Tidblad et al., 1998 and 2018; Graedel, 1996; Thebault et al., 2011 and 2012; Cole et al., 2011; Farrow et al., 1996). Moreover, MITReM (Multi-Ion Transport and Reaction Model) models can be considered, they being based on the addition of mass balances for all species in the electrolyte to the Poisson equation to solve the electrolyte potential and the species concentration distributions (Topa et al., 2012). A more general family of deterministic models is founded on the ANN (Artificial Neural Network) algorithm (Jančíková et al., 2013; Vera and Ossandón, 2014 and Panchenko and Marshakov, 2017), that defines a set of artificial neurons distributed according to a determined type of architecture. In the field of corrosion prediction, the variables which are generally chosen are the exposure time, concentration of atmospheric gas and environmental and thermodynamic parameters (i.e. relative humidity, ambient temperature and amount of rainfall).

### **The baseline experimental database**

#### ***Reference experimental data***

As indicated in Introduction, the research aims to develop a time-depending model to predict the corrosion depth propagation over a long period of time in wrought iron structures. The

literature models shortly described in the previous section fail to provide these data, since both the treated metal materials are not similar to wrought irons and they are mostly referred to corrosion from immersion.

However, a similar approach followed by other literature models (Guedes-Soares and Garbatov, 1999; Paik et al., 2004) is planned to be used to calibrate the numerical model through damage curves plotted according to experimental measurements. Unfortunately, the present authors could not carry out experiments on historical buildings, since such interventions would be very invasive on these existing structures. For this reason, some hypotheses to develop the study were formulated.

It is well known that, in terms of durability, the content in phosphorus closely affects the corrosion resistance of metals (Walker, 2002). It seemed, therefore, appropriate to focus the study on the content in phosphorus in order to place, in terms of durability, the wrought iron within traditional steels and to cope with the data gap. Studying the chemical composition of wrought irons, it was noted that its phosphorus content was estimated to be 0.1% (Boubée, 1880; Breymann and Koniger, 1925; Rossi, 1899; Walker, 2002) and that it ranged between 0.025 to 0.045% for mild/low carbon steels and it was equal to 0.16% for weathering steels (EN 10025, parts 2 and 5, 2004). Therefore, in this research phase, it was assumed that the wrought iron corrosion resistance in terms of durability is an average value between that of mild/low carbon steels and that of weathering ones (Degarmo et al., 2003).

This assumption is crucial for the present study and it was applied using the damage curves (corrosion depth propagation trends) developed by Fratesi (2002) for mild carbon and weathering steels in both marine and urban-industry atmosphere. In the Section titled A time-dependent damage model for predicting corrosion thickness wastage in wrought iron elements,

the mean curves (between mild steels and weathering steels) for both marine and industrial atmospheres, used to calibrate the new model herein proposed, are presented and discussed.

The damage curves developed by Fratesi specifically refer to buildings, located in Italian areas with marine and urban-industrial environmental conditions, made of mild/low carbon steels and weathering steels, recorded over a significant time interval (about 20 years). The curves cover a significantly long time interval of data, which is longer than or in line with models available in literature (Guedes-Soares and Garbatov, 1999; Paik et al., 2004). These curves were obtained by measurements performed on 20 Italian buildings, subjected to average environmental temperatures ranging from 0 to 30°C, over a period of about 20 years. Fratesi's damage curves are illustrated in Fig. 1, where the average material wastage thickness (or corrosion depth) is expressed as a function of time for both carbon mild steels (Fig. 1a) and weathering ones (Fig. 1b).

It is important to note that curves of Fig. 1 represent an interpolation of experimental data obtained by Fratesi. In addition, Fig. 1a shows a very high accelerating wastage rate for marine atmospheres and mild carbon steels. However, the comparison with literature models, and in particular with Melchers's model illustrated in the following, shows a similar tendency.

The two pairs of reference damage curves for mild carbon and weathering steels under marine and urban-industrial atmospheres were fitted on the basis of the least squares method by using a 3<sup>th</sup> order polynomial equation of the type  $y = p_1x^3 + p_2x^2 + p_3x + p_4$  ( $x = t$ ) in order to estimate the sample trend over a period of 125 years.

Table 1 gives the coefficients  $p_i$  of the above equation for the two materials (mild carbon steel and weathering steel) under the two considered environmental conditions (marine and urban-industrial). The uncertainty of the experimental data sample used for the method calibration is taken into account in the model herein proposed by considering the material wastage thickness

due to corrosion as a random variable with a normal distribution (Guedes-Soares and Garbatov, 1999). This hypothesis seems to be opportune when referred to the Fratesi's data sample, because he obtained his damage curves (Fig. 1) as a mean of the measurements. As a consequence, the experimental damage curves are considered as representative of the average values  $\mu_d(t)$ . Therefore, the mean of random values changing with a normal distribution (Guedes-Soares and Garbatov, 1999) is considered, while the standard deviation  $\sigma_d(t)$  is defined according to the study of Sarveswaran et al. (1998) on the basis of the relationship  $\sigma_d(t)=0.15\mu_d(t)$ .

Consequently, a sample with a size of corrosion depth equal to 100 (Desceliers et al., 2007) in both environmental conditions is generated by using a Monte-Carlo simulation (Hastings, 1970; Rose, 2014, Rizzo and Caracoglia 2018). In order to give a measure of the variability of the sample data set, the confidence interval (*CI*) and the tolerance one (*TI*) were estimated for randomly variable corrosion depth  $d_w(t)$ .

The confidence interval was estimated according to the following expression:

$$CI = \mu_d(t) \pm 1.96 \frac{\sigma_d(t)}{\sqrt{n_p}} \quad (1)$$

where 1.96 is the extent of the normal distribution for a degree of confidence equal to 95% and  $n_p = 100$ . *CI* is valid if the unknown error can be described by a normally distributed random variable.

Notoriously, the standard confidence interval equation relies on the population standard deviation. However, since the latter is generally unknown, it is replaced by the sample standard deviation. While this technically means that *CI* is an approximation of the confidence interval, it is a fairly accurate approximation for large samples (i.e.  $n_p \geq 30$ ). This is commonly referred to as the *large-sample* confidence interval (Walpole et al., 2002).

The  $TI$  intervals for each set of data points (i.e. 100 values of  $d_w(t)$ ) are estimated by the algebraic sum expressed through the following equation:

$$\mu d(t) \pm k \sigma d(t) \quad (2)$$

where the quantity  $k$  is the tolerance factor for a normal distribution.

In this study,  $k$  is set equal to 2.36, so that there is a 99% confidence that the calculated tolerance limits contain at least 95% of the measurements. The limiting confidence interval (i.e. 99%) must be added to the statement, since the bounds given by  $TI$  cannot be expected to contain any specified proportion (i.e. 95%) of all the time (Walpole et al., 2002). Fig. 2 shows  $CI$  and  $TI$  intervals and the randomly varying values, respectively for mild steels (Fig. 2a and 2c) and weathering steels (Fig. 2b and 2d).

### ***Comparison and critical remarks***

The  $d_w(T)$  trends estimated by the literature and code (i.e. EN ISO 9224 and EN 12500) models described in the Sections titled Literature overview were compared to both mild carbon and weathering steel damage curves developed by Fratesi for both marine and industrial-urban atmospheres (Fig. 1).

Fig. 3 shows a comparison between the Fratesi curves and the codes (i.e. EN ISO 9224 and EN 12500), respectively for mild carbon (Fig. 3a) and weathering (Fig. 3b) steels. In particular, these figures show a comparison between experimental data and codes ones for different corrosiveness categories (i.e. C1,...,C5). Fig. 3a shows that mild carbon in the marine atmosphere curve is between EN ISO 9224 C4 and C5 curves, whereas the mild carbon in the urban-industry atmosphere curves is between EN ISO 9224 C3 and C4 curves. However, as shown in Fig. 3b, the weathering curve in marine atmosphere is close to the EN ISO 9224 C3 one, whereas the weathering curve in urban-industry atmosphere is close to the EN ISO 9224 C2 one.

Fig. 4 shows a comparison between some literature 2<sup>nd</sup> level models, calibrated from experiments on immersed elements (i.e. Melchers, 1999; Guedes-Soares and Garbatov, 1999; Quin and Cui, 2003; Paik et al., 2004; Paik and Kim, 2012 and Mohd and Paik, 2013), and the Fratesi's curves. The comparison shows a closely high difference between literature and experimental curves, except than for Melchers (1999), who provided an acceptable estimation of the mild carbon curve (Fratesi, 2002) in marine atmosphere (Fig. 3a). Fig.4b shows that no literature models considered in the comparison give a satisfactory approximation of the weathering curves for both marine and urban-industry atmospheres.

Finally, Fig. 5 shows a comparison between literature models and results of experiments in atmosphere (i.e. Albrecht and Hall, 2003; Klimesmith, 2007; IPC and Vera and Ossandon, 2014). Fig. 4a shows a *good agreement* between the curve of the mild carbon in urban-industry atmosphere and the Albrecht and Hall's curve, whereas Fig. 5b shows a very great difference between literature models and experimental data. The only exception is represented by the weathering steel curve trend in marine atmosphere, that is acceptably reproduced by the Albrecht and Hall's curve, even if detected values are very different.

In order to measure the distance between the literature and codes' (i.e. EN ISO 9224 and EN 12500) models and Fratesi's curves (Fig. 1), the normalized root mean square error (NRMSE) is calculated in the window where curves are overlapped.

The relative errors have been also computed for both intervals *TI* and *CI* and they are defined as:

$$\varepsilon_{TI} = \left| \frac{d_w(t) - (\mu d(t) + k\sigma d(t))}{d_w(T)} \right|; \left| \frac{d_w(t) - (\mu d(t) - k\sigma d(t))}{d_w(T)} \right| \quad (3)$$

Similarly, the relative error between literature models and *CI* is defined as:

$$\varepsilon_{CI} = \left| \frac{d_w(t) - \left( \mu d(t) + 1.96 \frac{\sigma d(t)}{\sqrt{n_p}} \right)}{d_w(T)} \right| ; \left| \frac{d_w(t) - \left( \mu d(t) - 1.96 \frac{\sigma d(t)}{\sqrt{n_p}} \right)}{d_w(T)} \right| \quad (4)$$

It is important to specify that literature models are comparable with interpolating curves in the range  $0 \leq t \leq 20$  years only. In Table 2, the negative value means that models overestimate experimental values (Fratesi, 2002). The smallest error in absolute value are marked in grey.

Observing the percentage error listed in Table 2, it is worth noting that for marine atmospheres and mild carbon steels, the best mean error (i.e. -0.6%) compared to *TI* is given by Qin & Cui (2003). However, observing the maximum and minimum values of this error, it is clear that the range is very large (from about -70.6% to about 612.7%). The same is observed for the mean error compared to *CI*. The best value (3.5%) is given by Guedes-Soares and Garbatov (1999), but, similarly to *TI*, the range between the maximum and minimum error is very large. It ranges from about -36% to 83%. The best error range (from minimum to maximum) is given by Melchers (1999) for both *TI* and *CI*.

For urban-industrial atmospheres and mild carbon steels, the best mean error values are given by Albrecht and Hall (2003) for both *TI* and *CI*, respectively equal to about -18% and 23%. The Albrecht and Hall's model also gives a satisfactory value of NRMSE (i.e. 0.622), even if the best value, provided by Melchers, is equal to -0.41.

Results from the error analyses on marine and urban-industrial atmospheres for weathering steels are worse than those of mild carbon steels. For both marine and urban-industrial atmospheres, the EN ISO 9224 gives the best result in terms of mean error values, respectively equal to about 10% and -75%, compared to *TI*. EN ISO 9224 also gives the best value of the mean error for *CI* with urban-industrial atmospheres, approximately equal to -62%. Contrary, the best value for *CI* with marine atmospheres, equal to about 38%, is given by Albrecht and Hall (2003). However, the range between maximum and minimum errors is varying from -



2.2% to -46.5%. Low values of the NRMSE are due to the low values of the compared data. In conclusion, it was noted that, in spite of the trends illustrated in Figs. 3, 4 and 5, the values of the  $d_w(t)$  given by models and codes are *closely different* from the experimental data (Fratesi, 2002). It is important to note that the large differences observed between experimental data and literature and codes results are due to the lack of specific models for wrought iron elements.

### **A time-dependent damage model for predicting corrosion thickness wastage in wrought iron elements**

According to the hypothesis previously done in the Section titled The baseline experimental database, with the aim to calibrate a time-depending model specific for wrought iron elements, the mean curves between mild and weathering steels for both marine and urban-industrial atmospheres were achieved by interpolation.

Fig. 6 illustrates the interpolating damage curves for the two environmental conditions.

Table 3 gives the coefficients of the polynomial relationship that fitted the two environmental conditions (marine and urban-industrial) of the interpolating damage curves.

Neither of the previously described models are specific for historical buildings and the comparison between results obtained by Fratesi (2002) and those derived from the reference numerical models confirms this conclusion (Figs 3, 4 and 5).Based on this conclusion, this paper discusses a new corrosion depth model calibrated according to both the experimental damage curves given by Fratesi (2002) and the interpolating curves(Figs.6a and 6b).

The modelling method proposed to predict the corrosion phenomenon is, according to the 2<sup>nd</sup> level approach illustrated in Sections titled Introduction and Literature overview, based on the definition of damage curves for wrought iron elements, which provide the corrosion depth as a function of time (Guedes-Soares and Garbatov,1999; Melchers, 1998; Paik et al., 1998).

The reason of this choice, that is the use of a 2<sup>nd</sup> level approach, is mostly due to the lack of protection for long-time of historical buildings. Linear models, such as that developed by Southwell et al. (1979), and the models proposed by codes are specifically calibrated on frequent protection periods and are conceived for new structures. Contrary, specific models that consider long periods of maintenance absence are neglected by literature and codes.

In order to design a model adapted to historical buildings, Fratesi's damage curves were extended from 20 to 125 years using the interpolating curves and illustrated in Fig. 6. This extension was chosen in order to cover the entire period from the beginning of the experiments (1982) up to today. Summing up, Fig. 6 illustrates the interpolating and fitting damage curves for both marine and urban – industrial atmospheres.

The proposed model is obtained by analysis of the residues specifically estimated where the corrosion depth shows the maximum gradients. It allows for the definition of the formulation expressed through the relationship given in Eq. 5, which represents the rational function that gives the best approximation of the fitted data:

$$d_w(t) = \frac{p_1 t^2 + p_2 t + p_3}{t + p_4} \quad \forall t > 0 \quad (5)$$

where, according to the nomenclature used in the Section titled The baseline experimental database,  $d_w(t)$  is the loss of thickness in mm,  $t$  is the exposure time in years and coefficients  $p_{1...4}$  are constants for each single damage curves provided in Table 3.

Decay from the corrosion process was added to the Eq. 5 as a function of the number of coating cycles,  $\eta$ , and the time slot between two subsequent coatings  $\Delta T_{m,i}$ , supposed to be more persistent than the design life of the coating  $\tau_c$ , is assumed as equal to 10 years.

Consequently, Eq. 5 takes the form of the following equation:

$$d_c(t) = \sum_{i=1}^{\eta-1} [d_w(t)(\Delta T_{m,i} - \tau_c)] + d_w(t) \left( t - \sum_{i=1}^{\eta-1} (\Delta T_{m,i} - \tau_c) \right); \quad (6)$$

$$\forall t > 0; \forall n \in N; \Delta T_{m,i} > \tau_c$$

where  $d_c(t)$  is the function expressing the wastage thickness due to corrosion in case of a protected structure and  $d_w(t)$  is given by Eq.5 by adding  $\tau_c$ , (Guerrieri, 2005; Landolfo et al., 2007).

If  $\Delta T_{m,i}$  is constant (and assumed equal to  $\Delta T_m$ ), then Eq. 6 can be replaced as follows:

$$d_c(t) = (\eta - 1) \cdot d_w(\Delta T_m - \tau_c) + d_w\{t - [(\eta - 1) \cdot \Delta T_m + \tau_c]\}; \quad (7)$$

$$\forall t > 0; \Delta T_{m,i} > \tau_c$$

As it was shown before, the extended damage curves (Fig. 6) are considered as representative of the average values  $\mu_d(t)$ , intended as a mean of random values changing with a normal distribution, while the standard deviation  $\sigma_d(t)$  is fixed according to the Sarveswaran et al.'s study (1998).

Subsequently, a sample with a corrosion depth size equal to 100 (Desceliers et al., 2007) in both environmental conditions is generated by using a Monte-Carlo simulation (Hastings, 1970).

Fig. 7 shows an example of the probability density function (*pdf*) of the corrosion depth, under both marine and urban - industry environmental conditions, after 125 years using only one protection coating cycle ( $\eta = 1$  in Eqs. 6 and 7).

Fig. 8 shows the random variability of the corrosion depth for examined environmental conditions of the interpolating curves (Fig.6) extended to  $t = 125$  years while  $\tau_c$  equals 10 years. In the same figure the confidence interval (CI) of 95% is overlapped as well.

It is important to note that the model was calibrated on structures made of wrought iron and that evaluations extended to predict corrosion in other metals such as reinforcing steel in concrete are in progress, using the proposed model.

### **Application of the proposed corrosion wastage model to a case study**

The time-dependent corrosion wastage model described in the Section titled A time-dependent damage model for predicting corrosion thickness wastage in wrought iron elements is applied to a case study closely affected by corrosion phenomena. The case study is the Umberto I Gallery in Naples, the largest city in Southern Italy. It is a public shopping gallery, which was built between 1887 and 1892 as the corner stone of the Renaissance period. It was designed by Arch. Emanuele Rocco, who employed modern architectural elements reminiscent of the Vittorio Emanuele II Gallery in Milan (Carughi, 1996). The Gallery has a high and spacious cross-shaped structure. It is composed of four iron and glass-vaulted wings, about 25 m high, and a glass dome having diameter of 36 m and height of about 56 m, which is braced by 16 wrought iron ribs (Carughi, 1996). The gallery is located in a highly urbanised area of the city, characterised by elevated levels of city traffic and with a distance from the sea, as the crow flies, of just a few hundred metres. The structure is, therefore, subjected to corrosion due to the presence of both atmospheric pollution and marine aerosols (Guerrieri, 2005; Landolfo et al., 2007). For these reasons, a mixed atmosphere would be desirable for this case study. However, in order to apply the time-dependending model and to reduce the approximation of the Fratesi's curves (2002), analyses were repeated separately for both marine and urban-industrial atmospheres.

In addition, according to experiment results given by Landolfo et al. (2009), a hypothesis about both the presence of metal protection and the maintenance cycles has been made. In the case study, the protection covering was quantified from experiments carried out on original elements, as discussed in Landolfo et al. (2009). In this case, due to the absence of a maintenance plans, two cases were considered: one (Fig. 8) or five coating cycles during the construction life, assumed as equal to about 125 years (1892-2017). The single protective coating life,  $\tau_c$ , is assumed equal to 10 years. An additional hypothesis is that the corrosion depth is stopped during the coating life. The assumed hypotheses are validated by measurements of the corrosion depth directly taken on members placed outside and inside the Gallery (Landolfo et al., 2009). With reference to an original thickness equal to 3.0 mm, the thickness of outside elements decrease of 84%, whereas inside the structure this reduction is about 16% (Landolfo et al., 2009).

In this case study, the effect of the corrosion depth on the elements shows a reduction of the thickness of plates. As an example, in Fig. 9a, a 3 x 60 x 1000 mm plate taken from the Gallery (Landolfo et al., 2009), shows an increase of the coefficient  $c(t)$  defined as the ratio  $N/Af_y$ , where  $N$  is the axial force in Newton,  $A$  is the section plate area in  $\text{mm}^2$  and  $f_y$  is the yield stress of the material, assumed equal to 275 MPa. In this case, in a marine atmosphere  $c(t)$  is greater than 1 after 49 years, while on the contrary in an urban-industry atmosphere,  $c(t)$  is less than 1 after 115 years (i.e. always considering the protective coating life,  $\tau_c$ , approximately 10 years). Similarly, Fig. 9b shows a decrease of stiffness (i.e. axial stiffness) of the element. In a marine atmosphere, the stiffness is dangerously close to zero after 60 years.

With the aim to use corrosion damage as a yard stick for determining remaining mechanical strength in the material and also in the entire structure, static and dynamic analyses of the main structures (i.e. truss arches and beams) are carried out taking into account the time-dependency

of the corrosion depth according to the trend illustrated in Fig. 10. With reference to the curves shown in Fig. 7, a sample with a size of corrosion depth equal to 100 (Desceliers et al., 2007) in both environmental conditions and for five cycles of protection coating conditions is generated by using a Monte-Carlo simulation, through  $\mu_d(t)$  and  $\sigma_d(t)$ , in order to estimate the maximum and minimum corrosion depth values starting with the average curves illustrated in Fig. 10.

The maximum values are used both to estimate the maximum reduction of thickness in structural elements and, therefore, to perform finite element analyses on the structure.

*At the (time) age of analysis* (2017), the maximum value of the corrosion depth is 6.63 mm for the reduced marine condition and 2.37 mm for the urban-industrial situation.

Based on the 100 corrosion depth samples, the reduced sections (100 different sections) of all metallic structural elements (arches) are used to carry out static and dynamic analyses to estimate the limit condition of the structural collapse. The life limit state and the probability density functions of tensile or compression actions in the elements are then assessed.

The achieved results say that the remaining life of the construction is zero and equal to 49 years with a reduced marine atmosphere by using one cycle and five cycles (i.e. 1 cycle each 25 years) of protection coatings, respectively. On the other hand, under an urban-industrial atmosphere, the residual life is equal to 285 years and 585 years by applying one protection cycle and five protection cycles, respectively.

### **Concluding remarks**

In the current paper a time-dependent corrosion wastage model has been proposed to help in estimating the corrosion depth in historical metal buildings made of wrought iron. Average damage curves based on a sample of experimental measurements on 20 buildings for 20 years have been taken as a reference to calibrate the model, which is based on the hypothesis that

durability in historical wrought iron is similar to that of more recent mild carbon and weathering steels.

The reference curves have been taken(considered) as mean values of random variables with a normal distribution and a standard deviation of 0.15. The reference damage curves for two environmental conditions (marine and urban-industrial) and two different materials (mild carbon steel and weathering steel) have been interpolated, fitted and extended from 20 years to 125 years. The variability of protection coating life and its cycles of renovation have been added into the model as well. A comparison between experimental damage curves and some significant literature and codes models has been given and the percentage errors detected have been evaluated and discussed.

Finally, as an applicative example, the model has been applied to estimate the remaining life of the metal structural elements of the Umberto I Gallery in Naples, the largest city in the Southern Italy. In this case study, the computed time-depending model is applied with the aim to use corrosion damage as a yard stick to determine the remaining mechanical strength of the material. The prediction has been carried out by repeating static and dynamic analyses with different environmental conditions and different numbers of coating renovation cycles. In conclusion, the collapse condition of the structures has been identified by generating different sets of corrosion depth using the Monte-Carlo simulation based on the average damage curves. The achieved results have revealed that, under a reduced marine atmosphere, the gallery remaining life is null for one cycle of coating protection and is equal to 49 years using five protection cycles, which mean 1 cycle each 25 life years. On the other hand, best performances are obtained under an urban-industrial atmosphere, where the residual life of the construction is equal to 285 years and 585 years by using only one cycle and five cycles, respectively.

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## Appendix I: References overview.

### A.1 Melchers (1999)

Melchers (1999) applied the Southwell et al.'s linear equation (1979) to fit experimental data, so to obtain the expression given in the following equation:

$$d_w(t) = 0.076 + 0.038 \cdot t \quad (8)$$

where  $d_w(t)$  is the loss of thickness expressed in mm and  $t$  is the exposure time, measured in years, variable from 0 to 16.

The bi-linear equation given by Melchers (1999), as stated in Section 1, is valid for mild and low alloy steels in immersion conditions and it is directly based on experimental data.

$$\begin{cases} d_w(t) = 0.09 \cdot t & 0 \leq t < 1.46; \\ d_w(t) = 0.076 + 0.038 \cdot t & 1.46 \leq t < 16. \end{cases} \quad (9)$$

$$d_w(t) = 0.084 \cdot t^{0.823} \quad (10)$$

### A.2 Guedes-Soares and Garbatov (1999)

Guedes-Soares and Garbatov (1999)'s model on steel plated elements was improved by Qin and Cui (2003), who refined their previous study (2002), where they observed that, once the



coating system loses its effectiveness in a fixed time interval, a phenomenon of pitting corrosion starts and produces an increase of the damage rate. The model proposed by Guedes-Soares and Garbatov (1999) divided the corrosion process into three phases: in the first *cps* was effective, in the second *cps* was damaged (the phase started when the corrosion started) and in the third the corrosion process was terminated.

The model, updated in 2006, was focused on the variation of the thickness loss given by the following relationships:

$$\begin{cases} d_w(t) = d_\infty \left(1 - e^{-\frac{(1-\tau_c)t}{\tau_t}}\right) & t > \tau_c; \\ d_w(t) = 0 & t \leq \tau_c. \end{cases} \quad (11)$$

where  $d_\infty$  is the long-term thickness of the corrosion wastage (equal to 5 mm into the example used by Authors to calibrate the model),  $\tau_c$  is the coating life in years, equal to the time interval between the beginning of the surface painting and the time when its effectiveness is lost,  $\tau_t$  is the transition time, which may be calculated as  $\frac{d_\infty}{\tan\alpha}$ , where  $\alpha$  is the angle formed by the tangent at the origin (equal to  $15.2^\circ$  in the example provided by Authors), and  $t$  is the time varying from 0 to 25 years.

The model was calibrated assuming that the studied plate, coming from a ship deck, was loaded by wave-induced compressive loads assumed to follow the Weibull distribution. It was applied to the case study presented in Paik et al. (1998) related to bulk carriers. Since the time when anticorrosion coating loss its effectiveness is a random variable following a normal distribution (Cui et al., 1998), the reliability is conditional to the probability of the coating failure time. Assuming that the reliability of the plate can be associated with the generalised index of reliability, which is calculated from a multi-normal distribution, the Authors concluded that the relative corrosion depth  $d_w(t)$  was a function of time under the assumption that the corrosion thickness is approximated to both a linear function and an exponential one.

### ***A.3 Qin and Cui in 2003***

The model proposed by Qin and Cui in 2003 gave a generalization of both the Guedes-Soares and Garbatov's model and the Paik et al.'s one. The model assumed that the Weibull function describes the corrosion rate. It was described by the following equations:

$$\begin{cases} d_w(t) = d_\infty \left( 1 - e^{-\left(\frac{1-T_{st}}{\eta}\right)^\beta} \right) & T_{st} \leq t \leq T_L \\ d_w(t) = 0 & 0 \leq t \leq T_{st} \end{cases} \quad (12)$$

where  $\beta$  is the shape parameter of the Weibull distribution,  $T_{st}$  is the instant when pitting corrosion starts,  $d_\infty$  is the long-term thickness of the corrosion wastage and, finally,  $\eta$  is the number of coating cycles.

In the example provided by Qin and Cui,  $t$  varied between 0 and 25 years,  $d_\infty$  was equal to 1.64,  $T_{st}$  was equal to 1.38,  $\eta$  was equal to 9.19 and  $\beta$  was equal to 1.99. The corrosion rate estimation probabilistic models developed by Paik et al. from 1998 to 2013 need to be herein described, even if briefly, step by step.

### ***A.4 Paik et al. (1998 and 2004) and Mohd and Pak (2013)***

Paik et al. (1998, 2004), Paik and Kim 2013 on plates, and Mohd and Paik 2013 on offshore subsea oil well tube models, developed a probabilistic corrosion rate estimation model for strength assessment of longitudinal members of bulk carriers, based on available statistical data for corrosion of existing structures given by Loseth et al. (1994). In the same period, similarly, the Guedes-Soares and Garbatov (1999) model was based on non-linear general corrosion wastage in a plate, which was updated in 2006. The model was calibrated assuming that the plate was an element of a ship deck and that it was loaded by wave-induced compressive loads assumed to follow the Weibull distribution. It was applied to the case study presented in Paik et al. (1998) regarding bulk carriers. The model proposed by Paik et al. (1998) for the strength

assessment of longitudinal members of bulk carriers, as stated in Section 1, was based on statistical data available from Loseth et al. (1994) related to the corrosion of existing structures. The Paik et al.'s model (1998) was divided into two parts: the first part was associated to the coating life, which was assumed to follow the normal distribution, whereas the second one was connected to the corrosion progress, which was predicted on the basis of the Weibull distribution. This model assumed that corrosion starts immediately after coating effectiveness is lost.

Paik et al. (2004) described a model for low alloy carbon steel plates used for seawater ballast tanks in ship structures. This model, based on experimental data, gave three intervals, for ages of 5, 7.5 and 10 years, obtained from experimental corrosion data and related to 100% and 95% of samples, respectively:

$$d_w(t) = \begin{cases} 0.0466(t - 5.0) \\ 0.0579(t - 7.5) \\ 0.0823(t - 10.0) \end{cases} \quad (13)$$

$$d_w(t)_{95\%} = \begin{cases} 0.1469(t - 5.0) \\ 0.1938(t - 7.5) \\ 0.2894(t - 10.0) \end{cases} \quad (14)$$

where  $d_w(t)$  is the corrosion depth expressed in mm and  $t$  is the vessel age (variable between 0 and 27 years), respectively equal to  $t_r$  and  $t$  in the Paik et al.'s model. By comparing Eqs.1 and 7, it can be noted that the Paik et al.'s model was independent from the long-term thickness of the corrosion wastage.

The model proposed in Paik et al. (2004) was discussed in depth in the work developed by Paik and Kim (2012), who assumed that the Weibull distribution was the best function to represent the corrosion wastage progress. Moreover, they defined the loss of thickness (expressed in mm) as given in the following relationship:

$$d_w(t) = \left[ \frac{\alpha}{\beta} \left( \frac{t - \tau_c}{\beta} \right)^{\alpha-1} \right] e^{-\left( \frac{t - \tau_c}{\beta} \right)^\alpha} \quad (15)$$

where  $t = T - \tau_c$  is the exposure time (in years) after the breakdown of the coating,  $\tau_c$  is the coating lifetime,  $T$  is the age of the structure (variable from 11 and 27 years) and  $\alpha$  and  $\beta$  are shape and scale parameters of the Weibull function, respectively. Finally, they also gave a polynomial expression of the Weibull parameters, also valid for the model proposed by Mohd and Pak (2013) and for  $t$  variable from 5 to 23 years, as follows:

$$\alpha = 0.0020(T - \tau_c)^3 - 0.0994(T - \tau_c)^2 + 1.5604(T - \tau_c) - 6.0025$$

(Paik and Kim, 2012) (16)

$$b = 0.0004(T - \tau_c)^3 - 0.0248(T - \tau_c)^2 + 0.4793(T - \tau_c) - 2.3812$$

(Paik and Kim, 2012)

$$\alpha = -0.0228(T - \tau_c)^2 + 0.6183(T - \tau_c) - 0.9440$$

(Mohd and Paik, 2013)

$$b = 0.0014(t - \tau_c)^2 + 0.0047(t - \tau_c) + 0.2921$$

(Mohd and Paik, 2013)

#### ***A.5 Klimesmith (2007)***

Klimesmith et al. 2007 developed a model for the atmospheric corrosion of carbon steel, zinc, copper and aluminum, taking into account the effects of four environmental variables (TOW;

sulfur dioxide, salinity and temperature). The general form of the degradation model is the following:

$$d_w(T) = A * t^B \left(\frac{TOW}{C}\right)^D \cdot \left(1 + \frac{[SO_2]}{E}\right)^F \cdot \left(1 + \frac{[Cl]}{G}\right)^H e^{j(Te+Te_0)} \quad (17)$$

Where,  $T$  is the age of the structure, TOW is the time of wetness (h/year);  $SO_2$  is sulfur dioxide concentration ( $\mu\text{g}/\text{m}^3$ ); Cl is chloride deposition rate ( $\mu\text{g}/\text{m}^3/\text{day}$ );  $Te$  is the air temperature ( $^\circ\text{C}$ ) and finally, A, B, C, D, E, F, G, H, j and  $Te_0$  are empirical coefficients.

#### ***A.6 International Cooperative Programme (ICP) model***

The International Cooperative Programme (ICP) on “Effects on Materials, included Historic and Cultural Monuments” in the framework of the UN ECE convention on long range transboundary air pollution (Kucera 2004, Landolfo et al 2010). These functions have been formulated for different metal materials and are based on both long-term exposures and trend analysis based on repeated one-year measurements, taking also into account the unsheltered or sheltered (Equation 9) exposure. The degradation of metal over time is expressed by means of mass loss (ML) as a function of climatic parameters (Rh; T), gaseous pollutants ( $SO_2$ ,  $O_3$ ) and precipitation parameters (Rain,  $H^+$   $Cl^-$ ). In the following the means of mass loss (ML) for weathering steel unsheltered Eq.(18) and sheltered Eq.(19) are reported:

$$ML = 34 [SO_2]^{0.33} \exp\{0.020 Rh + f(T)\}t^{0.33} \quad (18)$$

$$ML = 8.2 [SO_2]^{0.24} \exp\{0.025 Rh + f(T)\}t^{0.66} \quad (19)$$

Where  $t$  is the exposure time;  $SO_2$  is the sulfur dioxide concentration ( $\mu\text{g}/\text{m}^3$ ),  $Rh$  is the relative humidity,  $T$  is the average annual temperature ( $^{\circ}\text{C}$ );  $f(T) = a(T - 10)$  when  $T < 10$   $^{\circ}\text{C}$ , otherwise  $b(T - 10)$ , with  $a$ ,  $b$  being constant values depending on the specific metal.

#### ***A.7 Vera and Ossandon (2014)***

Vera and Ossandon (2014) model is based on an ANN design to estimate and predict the corrosion rate of metals and alloys (in  $\mu\text{m}$ ), at different experiment stations as a function of: exposure time, measured in years; concentration of atmospheric, measured in  $\text{mg m}^{-2} \text{ day}^{-1}$ ; concentration of atmospheric chloride, measured in  $\text{mg m}^{-2} \text{ day}^{-1}$ ; relative humidity, in percentage; ambient temperature, measured in degrees centigrade; amount of rainfall, measured in mm. The model is based on training a radial basis ANN (i.e. a type of neural network whose response or output is a function of the distance to a determined centre point) to precisely fit known or observed data. Once the weight values have been set, depending on the respective networks and designed for each station, it is possible to predict values for the corrosion rate of metals and alloys as a function of the above input variables. Networks have a two-layer structure: a hidden layer and an output layer. All the connections on the two-layer network are in a forward direction. In addition, as long as the neurons on the hidden layer have radial and nonlinear transfer functions, the neurons on the output layer make linear combinations with the corresponding activation of neurons on the hidden layer.

#### ***A.8 EN ISO 9224***

International Standard ISO 9224 specifies the long term corrosion rates for standard structural materials in the five corrosivity classes C1–C5. According to the Standard, the average corrosion rate of each material follows a bi-linear law. During the first 10 years, the corrosion depth is given by the formula:

$$d_w(t)_{10} = rav \cdot t \quad (t < 10 \text{ years}) \quad (20)$$

where  $d_w(t)_{10}$  is the corrosion depth after the first 10 years of exposure (micrometers);  $rav$  is the average corrosion rate (micrometers per year). After 10 years of exposure, the corrosion rate is assumed to be constant with time and the thickness loss is given by the formula:

$$d_w(t)_i = rav \cdot 10 + r_{lin} \cdot (t - 10) \cdot t \quad (t \geq 10 \text{ years}) \quad (21)$$

where  $d_w(t)_i$  is the corrosion depth for the considered time interval (micrometers);  $r_{lin}$  is the steady state corrosion rate (micrometers per year). The standard provides the guiding values of both  $rav$  and  $r_{lin}$  for carbon steel, weathering steel, zinc, copper and aluminum.

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